

Study on working peculiarities of glue laminated beams under conditions of slanting bending

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Abstract. Current rules do not take into account the peculiarities of glue laminated beams under the conditions of slanting bending. In fact, the rules do not consider the real change in the stress-strain behavior of such elements upon the load change from its initial application till fracture under the conditions of slanting bending due to the fact that material (wood) behavior in the construction is currently assumed as nominally elastic. What is more, this issue is not sufficiently studied either theoretically or experimentally.

This article is devoted to the study of the working peculiarities of glue laminated beams made of glued pine boards under the conditions of slanting bending.

The main issues are as follows, the description of research methodology and experimental prototypes, analysis and processing of the research results, working peculiarities of the material as part of the glue laminated wooden beams under the conditions of slanting bending.

The article presents the results of experimental studies on the bearing capacity and deformity of glue laminated beams under the conditions of slanting bending. The results will give the opportunity to design wooden constructions taking into account the profound knowledge on material capacity as well as the working peculiarities of its components, which will lead to a more practical cross section of structural components.

Investigation of the process of laminar deformation upon the section height and indication of the peculiarities of the stress-strain behavior of these layers due to the stress of physical force un-



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der the conditions of slanting bending will allow to perform more reliable calculations of stress-strain behavior and design of glue laminated beams at different stages.

Keywords: wood, bearing capacity, slanting bending, strain, stress, stiffness.

INTRODUCTION

Wood is not only one of the oldest construction materials, but also an important material to be used both in the present and future. Considerable increase in its use in the construction industry is an irresistible argument to this. Modern building wood has several benefits, such as light weight, easy manufacture, firmness and relatively cheap cost, constant

ing. Serviceability check of the strain gauges was performed in advance. The first check was performed prior to gluing by at ester; the second was performed after gluing and soldering by the computer software. The SIIT-3 strain-gauge station was responsible for sensor reading. Deflectometers were set on the supports in the points of load application and in the middle of the span to calculate deflections along the Y-Y and Z-Z directions. All the devices that were used in the experiment were preliminary approved by the State certification licensing authority.

The values of deflection of the beams were recorded prior to the load application which ranged to 80% of the assumed devastating load since further load application could lead to immediate fracture of the prototypes and, consequently, to the eventual fracture of the measurement instrumentation.

RESULTS

Laminated wooden beams GLB-1 and GLB-2 were tested at the slope angle of 10° , GLB-3 and GLB-4 beams were tested at the slope angle of 25° .

Five minute delay was permitted at each stage of the load application to take the readings from all devices.

During the experiment, control was executed over the level of load application permit-

ting critical deflection values [12, 13]. According to [12], the maximum allowed deflection of the beams is $1/150 l$, where l is the span of a beam. In our experiment, the maximum allowed deflection of the beams is as follows:

$$w_{fin} = \frac{l}{150} = \frac{2700}{150} = 18 \text{ mm} \quad (1)$$

In the first place, the deflections along the Y-Y and Z-Z directions were calculated [13].

Afterwards load-strain diagrams were drawn, which were calculated by the formula:

$$w = \sqrt{w_y^2 + w_z^2} \quad (2)$$

where: w_y is the deflection along the Y-Y direction, mm; w_z is the deflection along the Z-Z direction, mm.

As reflected by the graph (Fig.2), at different slope angles the critical deflection was achieved at different values of the moment. The critical deflection for GLB-1 and GLB-2 beams occurred at 7,2 kNm and 8,28 kNm moment, while for GLB-3 and GLB-4 beams at 10,35 kNm and 10,8 kNm respectively. This can be explained by a greater slope angle of the beam, since the secant line of the force direction creates the cross-section which is vertically more expanded at a slope angle of 25° in comparison with the cross section at a slope angle of 10° .

All prototype beams GLB-1 – GLB-4 frac-

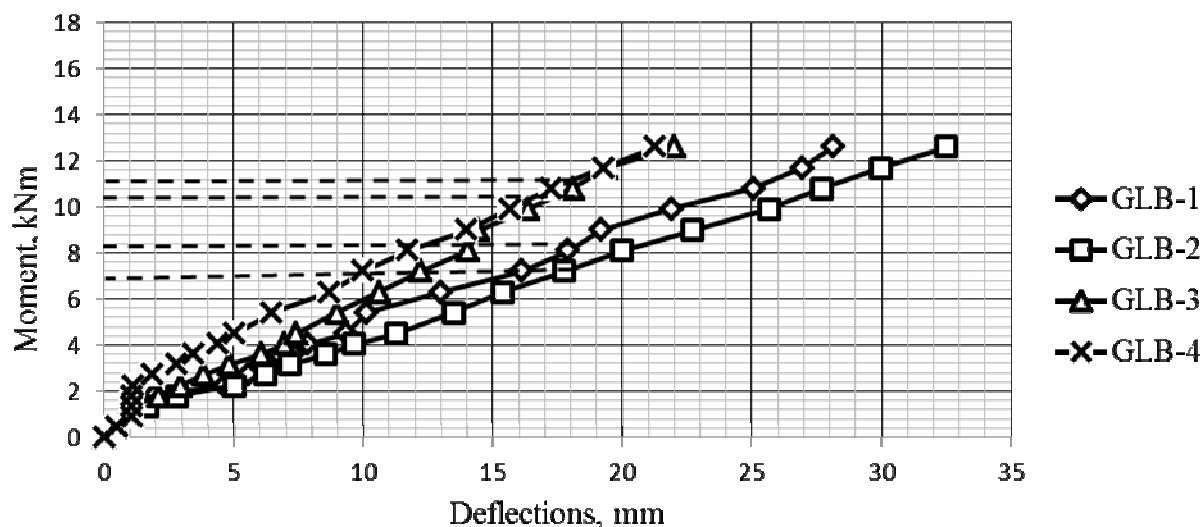


Fig.2. The full-scale deflections of glue laminated beams

tured in the area of pure bending and so did the beams at direct bending [14 – 17]. The first cracking of the beams occurred when tested at the slope angle of 10° at 30 kN moment, whereas at the slope angle of 25° it occurred at 32 kN. Afterwards, the rapid increase in strain occurred under the further increase of the load application, which lead to fracture of the unit [18 – 22], which started from the marginal board in the area of pure bending and was followed by crack propagation towards the supports. The fracture behavior of the GLB-1 and GLB-3 beams is shown in Fig.3 and in Fig.4.

The breaking moment for glue laminated beams GLB-1 and GLB-2 at the slope angle of 10° amounts 17,1 kNm and 17,33 kN, while this figure amounts 17,55 kN and 17,82 kN accordingly for the GLB-3 and GLB-4 beams at a slope angle of 25° .

The position of the neutral line at different levels of stress in a normal section in the area of pure bending of the askew-bended elements made from glue laminated wood was also determined by means of linear interpolation of the sensor readings. This method of determining the neutral line is the most conventional.



Fig.3. Fracture of the GLB-1



Fig.4. Fracture of the GLB-3

The neutral line of all the prototype glue laminated wooden beams changed its position towards increasing the area of the compressed zone under the increased load, simultaneously its slight twisting could be observed. The position of the neutral line for GLB-1 and GLB-3 beams under the different load levels are shown in Fig.5 and in Fig.6.

On processing the results obtained from the strain gauges, the deformation diagram was carried out for the deformation of the layers of wood in both the compressed and stretched areas of the prototype beams under the loads accommodation from the start of the load application and towards the loads on the verge of fracture while working under the conditions of slanting bending at the slope angles of 10° and 25° .

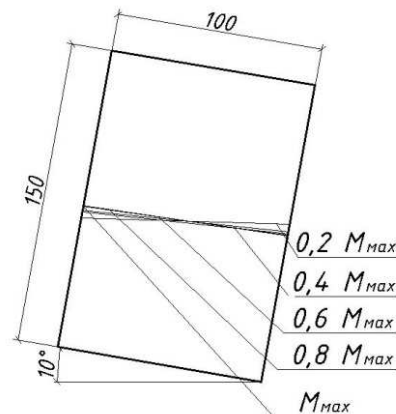


Fig.5. Change in the position of the neutral line of the GLB-1

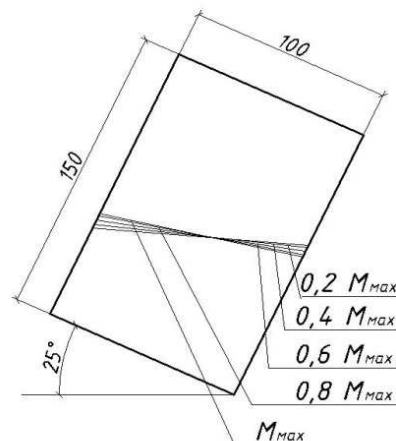


Fig.6. Change in the position of the neutral line of the GLB-3

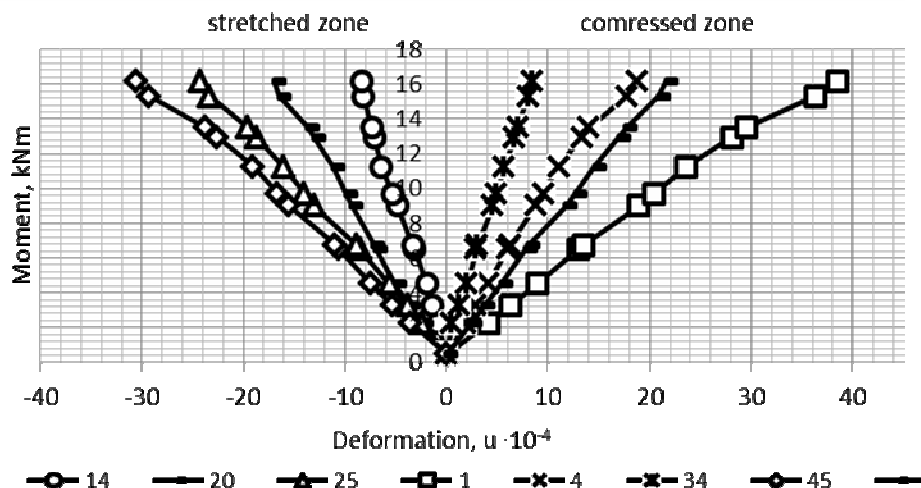


Fig.7. The diagram of GLB-1 deformation under the unsymmetrical bending at a slope angle of 10°

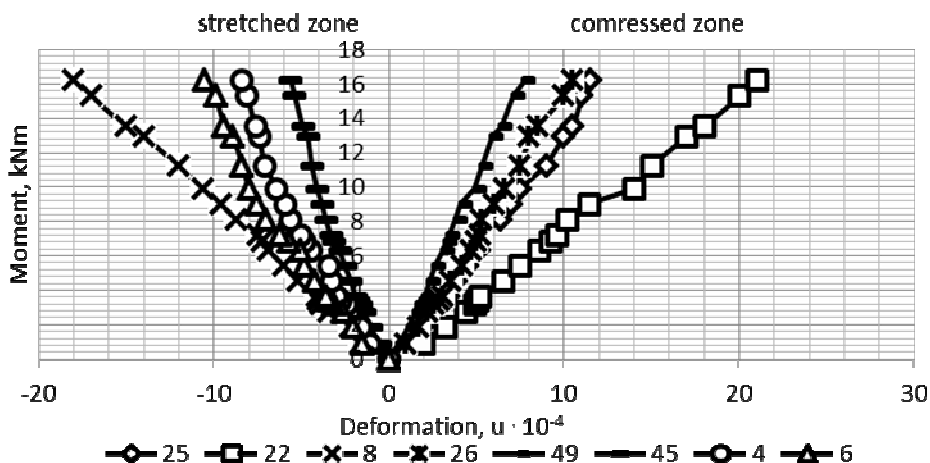


Fig.8. The diagram of GLB-3 deformation under the unsymmetrical bending at a slope angle of 25°

These graphs are shown in Fig.7 and in Fig.8. The number of strain gauges and their location on glue laminated beams are indicated to the right of the diagrams.

Deflectometers were positioned in the areas of load application in order to determine the value of the modulus of elasticity of glue laminated beams [16, 23]. The modulus of elasticity was calculated according to the formula:

$$E_x = \frac{F \cdot l^3}{548 \cdot I \cdot (w_c - w_N)}, \quad (3)$$

where F – is the load affecting the beam, kN;
 l – is the calculated span of the beam, m;
 I – is the cross-sectional moment of inertia of the prototype beam, m³;
 w_c – is the beam deflection in the middle of the span, mm;
 w_N – is the beam deflection at the point of load application, mm.

The values of the elastic modulus at different levels of load application are shown in Table 1.

The elastic modulus of the glue laminated wood for GLB-1 beam amounted 13814 MPa, for GLB-2 beam – 13945 MPa.

Table.1. The elastic modulus of the beams at different load application levels

Beam	Load Application Level	Elastic Modulus, E, MPa
GLB-1	0,2 M _{max}	18476
	0,4 M _{max}	14105
	0,6 M _{max}	12091
	0,8 M _{max}	10585
GLB-2	0,2 M _{max}	18363
	0,4 M _{max}	14367
	0,6 M _{max}	12364
	0,8 M _{max}	10689

CONCLUSIONS

On the basis of experimental works the following conclusions can be drawn:

- the moment of fracture for glue laminated beams GLB-1 and GLB-2 at the slope angle of 10° amounted 17,1 kNm and 17,33 kN, for GLB-3 and GLB-4 beams at the slope angle of 25° – 17,55 kN and 17,82 kN, respectively
- GLB-1 beam reached critical deflection values at the moment of 8.28 kNm, GLB-2 beam – at 7,2 kNm, GLB-3 beam – at 10,35 kNm, GLB-4 beam – at 10,8 kNm;
- the value of the elastic modulus of the GLB-1 amounts 13814 MPa, for GLB-2 – 13945 MPa;
- fracture behavior of the glue laminated wooden beams under the unsymmetrical bending at the slope angles of 10° and 25° was determined;
- the position of the neutral line of the beams under the slanting bending was determined and the method of its defining was suggested;
- the change in position of the neutral line of the askew-bended beams at the slope angles of 10° and 25° from the initial load application till fracture was specified.

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Исследование работы балок из клееной древесины в условиях косоугольного изгиба

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Аннотация. Действующие нормы не учитывают особенностей работы балок из клееной древесины при косоугольном изгибе. Также нормы не учитывают реальное изменение напряженно-деформированного состояния таких элементов при изменении нагрузки от начала приложения до разрушения в условиях косоугольного изгиба, поскольку в настоящее время работа материала (древесины) в конструкциях принимается условно упругой, и кроме того, этот вопрос как теоретически, так и экспериментально еще недостаточно изучен.

Статья посвящена изучению особенностей работы деревянных балок из клееных сосновых досок при косоугольном изгибе. Основными вопросами являются следующие: описание методики исследований опытных образцов, анализ и обработка полученных результатов исследований, изучение особенностей работы материала в составе дощатоклееных деревянных балок в условиях косоугольного изгиба. В статье приведены результаты экспериментальных исследований несущей способности и деформативности балок из клееной древесины в условиях косоугольного изгиба. Результаты позволяют проектировать конструкции из древесины, используя более полный учет возможностей материала и особенностей работы элементов, что приведет к более экономному подбору поперечного сечения элементов конструкций.

Исследование процесса послойного деформирования по высоте сечения и установление особенностей напряженно-деформированного состояния этих слоев под действием нагрузки при работе на косоугольный изгиб позволит с большей достоверностью проводить расчет на разных стадиях напряженно-деформированного состояния при проектировании клееных деревянных балок.

Ключевые слова: древесина, несущая способность, косоугольный изгиб, деформации, напряжения, жесткость.